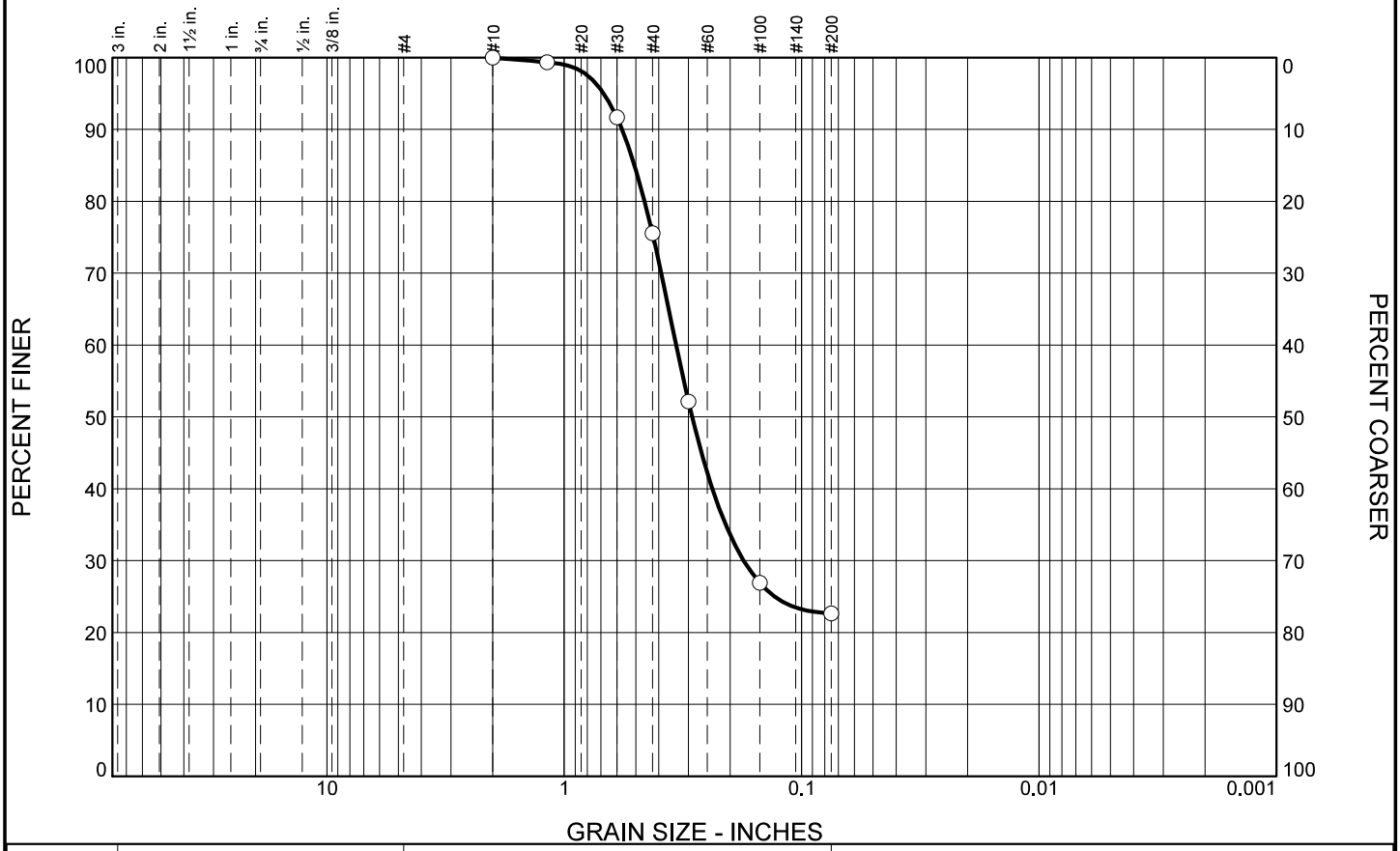
Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0.0	0.0	0.0	0.0	24.4	53.0	22.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#16	99.4		
#30	91.7		
#40	75.6		
#50	52.2		
#100	26.9		
#200	22.6		

* (no specification provided)

Soil Description

Orange, Silty SAND

Atterberg Limits

PL= NP LL= NP PI= NP NM= 13.9

Coefficients

D_{90}= 0.5709 D_{85}= 0.5057 D_{60}= 0.3378
D_{50}= 0.2895 D_{30}= 0.1754 D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= SM AASHTO= A-2-4(0)

Remarks

Location: I-4
Sample Number: S-2 **Depth:** 2.0-4.0 feet **Date:** 11/3/2021

GEO-TECHNOLOGY ASSOCIATES, INC.
21133 Sterling Avenue, Suite 7
Georgetown, DE 19947

Client: GED S. Main Dist. LLC
Project: PODS Bridgeville

Project No: 31211931 **Figure**

Tested By: K. Kershaw **Checked By:** T. Caraway

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, you can benefit from a lowered exposure to problems associated with subsurface conditions at project sites and development of them that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed herein, contact your GBA-member geotechnical engineer. Active engagement in GBA exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Understand the Geotechnical-Engineering Services Provided for this Report

Geotechnical-engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical-engineering services is typically a geotechnical-engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical-engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

Geotechnical-Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client.

Likewise, geotechnical-engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical-engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If you are the least bit uncertain* about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. *Read and refer to the report in full.*

You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept*

responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface using various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are <u>not</u> final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- confer with other design-team members;
- help develop specifications;
- review pertinent elements of other design professionals' plans and specifications; and
- be available whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for information purposes only.* To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are <u>Not</u> Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures.* If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer's services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, ***proper implementation of the geotechnical engineer's recommendations will <u>not</u> of itself be sufficient to prevent moisture infiltration. Confront the risk of moisture infiltration*** by including building-envelope or mold specialists on the design team. ***Geotechnical engineers are <u>not</u> building-envelope or mold specialists.***

GEOPROFESSIONAL BUSINESS ASSOCIATION

Telephone: 301/565-2733
e-mail: info@geoprofessional.org www.geoprofessional.org

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GEO-TECHNOLOGY ASSOCIATES, INC.

GEOTECHNICAL AND
ENVIRONMENTAL CONSULTANTS

A Practicing Geoprofessional Business Association Member Firm

March 11, 2022

GED S. Main Dist. LLC
687 Old Willets Path
Hauppauge, New York 11788

Attn: Mr. Gary Krupnick

Re: Report of Supplemental SWM Subsurface Exploration
PODS Bridgeville
Bridgeville
Sussex County, Delaware

Ladies & Gentlemen:

In accordance with our agreement dated February 9, 2022, Geo-Technology Associates, Inc. (GTA) has performed a supplemental subsurface exploration for the proposed stormwater management (SWM) area for the above referenced project. The purpose of the subsurface exploration was to evaluate the estimated seasonal high groundwater elevation; discuss suitability of the subsoils to facilitate infiltration practices at selected test locations; and to present our recommendations regarding SWM facility construction. Plans titled *PODS STORAGE FACILITY - Proposed Soil Borings Test Locations* prepared by Becker Morgan Group (BMG) and revision dated January 4, 2022 and our previous reports dated December 3 and 7, 2021, were referenced for this report. The results of our supplemental SWM subsurface exploration are summarized below.

Referring to the Site Location Map and the Exploration Location Plan included as Figures 1 and 2, respectively, in Appendix A, the subject property is located along the east side of South Main Street approximately 200-feet north of Rifle Range Road in Bridgeville, Delaware. The study area consists of an agricultural field. The project site is relatively flat with the ground surface at the exploration locations generally ranging between Elevation 41 and 42 Mean Sea Level (MSL).

According to the Geologic Map of Seaford West and Seaford East Quadrangles (2015) published by the Delaware Geological Survey, the site is within the Coastal Plain Physiographic Province. Coastal Plain sediments below the surficial deposits exposed in the site area were generally deposited in commonly estuarine environments of the Tertiary geologic age. The Late Pliocene deposits designated as the Beaverdam Formation. Sediments of the Beaverdam Formation typically consist of "…very coarse sand with pebbles to silty clay. The predominant lithologies at the land surface are white to mottled light-gray and reddish-brown, silty to clayey,

fine to coarse sand. Laminae and beds of very coarse sand with gravel are common." Please refer to the publication for additional information.

From review of the USDA Soil Survey, the soils predominately conform to the Hambrook sandy loam complex, (0 to 2 percent slopes). The soils map information is attached.

From review of the attached Monthly Groundwater Depth for Pe54-51, Columbia Aquifer, taken from the Delaware Geological Survey website, the groundwater depth at Well Pe54-51 (west of Millsboro, Delaware), was above normal when the borings were performed in February 2022.

GTA performed five Standard Penetration Test (SPT) borings, designated as SWM-1 through SWM-5, to depths of 14 feet below the ground surface. Temporary piezometers were placed in each test hole and longer-term water readings were taken twelve days after completion. The piezometers were removed after the long-term readings. The exploration locations were selected and staked with elevations determined by BMG. Relative locations are shown on the attached Exploration Location Plan. The exploration locations indicated on the plan should be considered approximate.

The soils were visually classified in accordance with the Unified Soil Classification System (USCS) and the United States Department of Agriculture (USDA) classification system. The borings encountered a 4 to 10-inch-thick surficial topsoil layer. Below the topsoil layer, the explorations generally encountered native soils visually classified as consisting of Silty SANDs (USCS: SM; USDA: Sandy Loam), Poorly-graded SANDs (SP; Sand), Poorly-graded SAND with Silt (SP-SM; Loamy Sand) and Clayey SANDs (SC; Sandy Clayey Loam). The relative densities of the granular soils were very loose to medium dense based on SPT N-values of 2 to 12 blows per foot (bpf).

At Boring SWM-1, Lean CLAY (CL; Clay Loam) was encountered at an approximate depth of 9 to 12 below ground surface. The consistency of the fine-grained soils was soft to stiff based on an SPT N-value of 4 to 11 bpf.

GTA's estimate of the seasonal high groundwater level at the borings is based upon soil coloring, mottling and/or saturation. The results of the groundwater level readings and GTA's opinion of the estimated seasonal high groundwater depth are summarized as follows:

GROUNDWATER DATA SUMMARY

Exploration No.	Existing Ground Surface Elevation (MSL)	Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Groundwater at Completion	Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Groundwater At Twelve Days After Completion	*Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Estimated Normal Seasonal High Groundwater
SWM-1	EL 41.3	13.0 / EL 28.3	12.6 / EL 28.7	6 / EL 35
SWM-2	EL 40.8	8.3 / EL 32.5	9.2 / EL 31.6	6 / EL 35
SWM-3	EL 40.8	10.2 / EL 30.6	9.4 / EL 31.4	6 / EL 35

Exploration No.	Existing Ground Surface Elevation (MSL)	Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Groundwater at Completion	Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Groundwater At Twelve Days After Completion	*Depth Below Existing Ground Surface (ft.)/ Elevation (MSL) to Estimated Normal Seasonal High Groundwater
SWM-4	EL 41.7	11.0 / EL 30.7	10.8 / EL 30.9	7 / EL 35
SWM-5	EL 41.0	8.5 / EL 32.5	9.7 / EL 31.3	6 / EL 35

*Seasonal high groundwater estimate based upon observed soil mottling, color and/or saturation and should be considered approximate.

The groundwater levels can be expected to fluctuate with seasonal changes, precipitation, and other factors such as development activity. Additionally, perched water conditions develop in granular soils overlying fine-grained soils during the "wet season" as well as during periods of precipitation. Please refer to the idealized Subsurface Profile and exploration logs provided in the attachments for further information.

A selected sample obtained from the borings was tested for grain-size analysis, Atterberg Limits and natural moisture content. The grain-size analysis and Atterberg Limits testing were performed to determine the Unified Soil Classification System (USCS) designations for the soil. The results of testing are as follows:

SUMMARY OF LABORATORY TESTING

EXPLORATION NO.	DEPTH (FT.)	USCS CLASSIFICATION	LL (%)	PI (%)	NMC (%)
SWM-1	4 – 6	Silty SAND (SM)	NP	NP	14.2

Note: LL=Liquid Limit PI=Plastic Index NP=Non-plastic NMC=Natural Moisture Content

Five single ring, 12-inch diameter, falling head infiltration tests were also performed at locations offset from the borings. The infiltration test holes were pre-soaked prior to the falling head tests. Each test location was charged with a six-inch head and the water level drop was recorded at 15 minutes or less intervals. The results of the infiltration tests, test depths, estimated seasonal high groundwater and soil type at each test location are summarized as follows:

SUMMARY OF INFILTRATION DATA

TEST NO.	EXISTING GROUND SURFACE ELEVATION (MSL)	*DEPTH BELOW EXISTING GROUND SURFACE (FT.)/ ELEVATION (MSL) TO ESTIMATED SEASONAL HIGH GROUNDWATER	INFILTRATION TEST DEPTH BELOW EXISTING GROUND SURFACE (FT)/ ELEVATION (MSL)	**INFILTRATION TEST INFILTRATION RATE (IN/HR)	ESTIMATED VISUAL USDA SOIL CLASSIFICATION
SWM-1	EL 41.3	5 / EL 36	4.2 / EL 37.1	2.0	Sandy Loam
SWM-2	EL 40.8	5 / EL 36	2.8 / EL 35.8	2.0	Sandy Loam
SWM-3	EL 40.8	6 / EL 36	5.0 / EL 35.8	1.5	Sandy Loam
SWM-4	EL 41.7	5 / EL 36	6.5 / EL 35.2	0.5	Loamy Sand
SWM-5	EL 41.0	5 / EL 36	4.8 / EL 36.2	2.3	Sandy Loam

*Seasonal high groundwater estimate based upon observed soil mottling, color and/or saturation and should be considered approximate.
** Infiltration tests performed in general accordance with ASTM D-5126.

The guidelines established in the *Delaware Post Construction Stormwater BMP Standards & Specifications*, dated February 2019 indicate that the minimum infiltration rate for all runoff reduction and infiltration practices is one-inch per hour. Also, a vertical separation of at least two-feet from the seasonal high groundwater elevation or limiting layer is required for all infiltration practices unless an underdrain is provided.

Based upon the average infiltration rates and using a factor of safety of 2.0, GTA recommends the following for infiltration rate for the SWM design.

SUMMARY OF INFILTRATION SUITABILITY

SWM AREA	PREDOMINANT USDA SOIL TYPE	SUITABILITY OF LOCATION FOR INFILTRATION PRACTICE	RECOMMENDED FACILITY BOTTOM RANGE OF ELEVATION (MSL)	RECOMMENDED MAXIMUM DESIGN INFILTRATION RATE (IN/HR)*
SWM-1 through SWM-5	Sandy Loam	Suitable	EL 37 to 38	1.6

* FS=2.0

GTA recommends placing the facility bottom through less permeable Sandy Clay Loam and Clay Loam and into Sandy Loam layers. To facilitate placement in Sandy Loam layers, the actual facility bottom elevation will vary and should be adjusted as required in the field based upon the observed conditions at the time of construction. In areas where less permeable soil including Sandy Clay Loam and Clay Loam is encountered at the basin bottom up to within two feet below the facility bottom, the soils should be over excavated and replaced with ASTM C33 Concrete Sand.

GTA recommends that infiltration facilities be excavated using a track-mounted excavator, which will generally eliminate the need to operate equipment directly on the subgrade.

Post-construction infiltration testing should be provided and the groundwater depth observed within proposed SWM facilities with the results reported to the site engineer for conformance with the facility design parameters. The earthwork contractor should anticipate that the post-construction infiltration testing will take two days to complete once the facility bottom is exposed.

If a portion of a SWM facility is to be used for a temporary sediment basin, GTA recommends excavating the basin to two feet above the planned pond bottom and limiting the footprint of the temporary basin, if practical, to minimize degradation (e.g., decreased permeability) of the infiltration facility basin.

If wet pond construction is considered, a pond liner will be required to maintain pool levels, depending upon pond bottoms. Depending upon conditions observed in the field at the time of construction and to assist in maintaining the wet pond levels during extended dry

weather, an irrigation well may be considered to provide a supplemental water source for the pond due to potential loss of pond water levels mostly due to evaporation and during times of below normal average groundwater conditions. A sufficient quantity of USCS SC and CL materials may be available on site to be used for a pond liner. If enough SC and CL materials are not available GTA recommends a Geosynthetic Clay Liner (GCL; Bentonite matrix) or an appropriate PVC liner with relief valves may be used as a pond liner. Both types of liners will need to be provided with a 1-foot thick granular soil cover. The GCL or PVC liners should be installed in accordance with manufacturer's recommendations. On-site granular soils are considered suitable for use as a pond liner cover material if they are dried to near optimum. Pond liner cover materials should meet AASHTO classification designation A-2-4 or more granular and be approved by GTA.

Structural fill should be constructed in maximum 8-inch loose lifts and compacted to 95 percent of the maximum dry density as determined by ASTM D-698 (AASHTO T-99). If practical, GTA recommends reinforced concrete pipe be used as the principal spillway pipe. Also, a concrete cradle and anti-seep collar should be provided for the spillway pipe.

For wet pond construction, water levels may be above at least a portion of the pond bottom level during construction. The contractor should be prepared to stabilize and dewater pond excavations. Subgrades excavated below the water table will be prone to instability and softening.

All SWM pond construction should conform to *Delaware Conservation Practice Standard Pond Code 378* and *Code 521*, latest editions and *Delaware Sediment and Stormwater Regulations*, latest edition, as applicable.

Limitations

This report, including all supporting exploration logs, field data, field notes, estimates, and other documents prepared by GTA in connection with this project, has been prepared for the exclusive use of GED S. Main Dist. LLC. pursuant to the agreement between GTA and GED S. Main Dist. LLC dated February 9, 2022, and in accordance with generally accepted engineering practice. All terms and conditions set forth in the Agreement are incorporated herein by reference. No warranty, express or implied, is given herein. Use and reproduction of this report by any other person without the expressed written permission of GTA and GED S. Main Dist. LLC is unauthorized and such use is at the sole risk of the user.

The analysis and recommendations contained in this report are based on the data obtained from limited observation and testing of the encountered materials. Explorations indicate soil and groundwater conditions only at specific locations and times and only to the depths penetrated. They do not necessarily reflect strata variations that may exist between the exploration locations. Consequently, the analysis and recommendations must be considered preliminary until the subsurface conditions can be verified by direct observation at the time of construction. If

variations in subsurface conditions from those described are noted during construction, recommendations in this report may need to be re-evaluated.

In the event that any changes in the nature, design, or location of the facilities are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report are verified in writing. Geo-Technology Associates, Inc. is not responsible for any claims, damages, or liability associated with interpretation of subsurface data or reuse of the subsurface data or engineering analysis without the expressed written authorization of Geo-Technology Associates, Inc.

The scope of our services for this geotechnical exploration did not include any environmental assessment or investigation for the presence or absence of wetlands, or hazardous or toxic materials in the soil, surface water, groundwater or air, on or below or around this site. Any statements in this report or on the logs regarding odors or unusual or suspicious items or conditions observed are strictly for the information of our Client. The subject matter of this report is limited to the facts and matters stated herein. Absence of a reference to any other conditions or subject matter shall not be constructed by the reader to imply approval by the writer.

We appreciate the opportunity to be of assistance on this project. Should you have any questions or require additional information, please contact our office at (302) 855-9761.

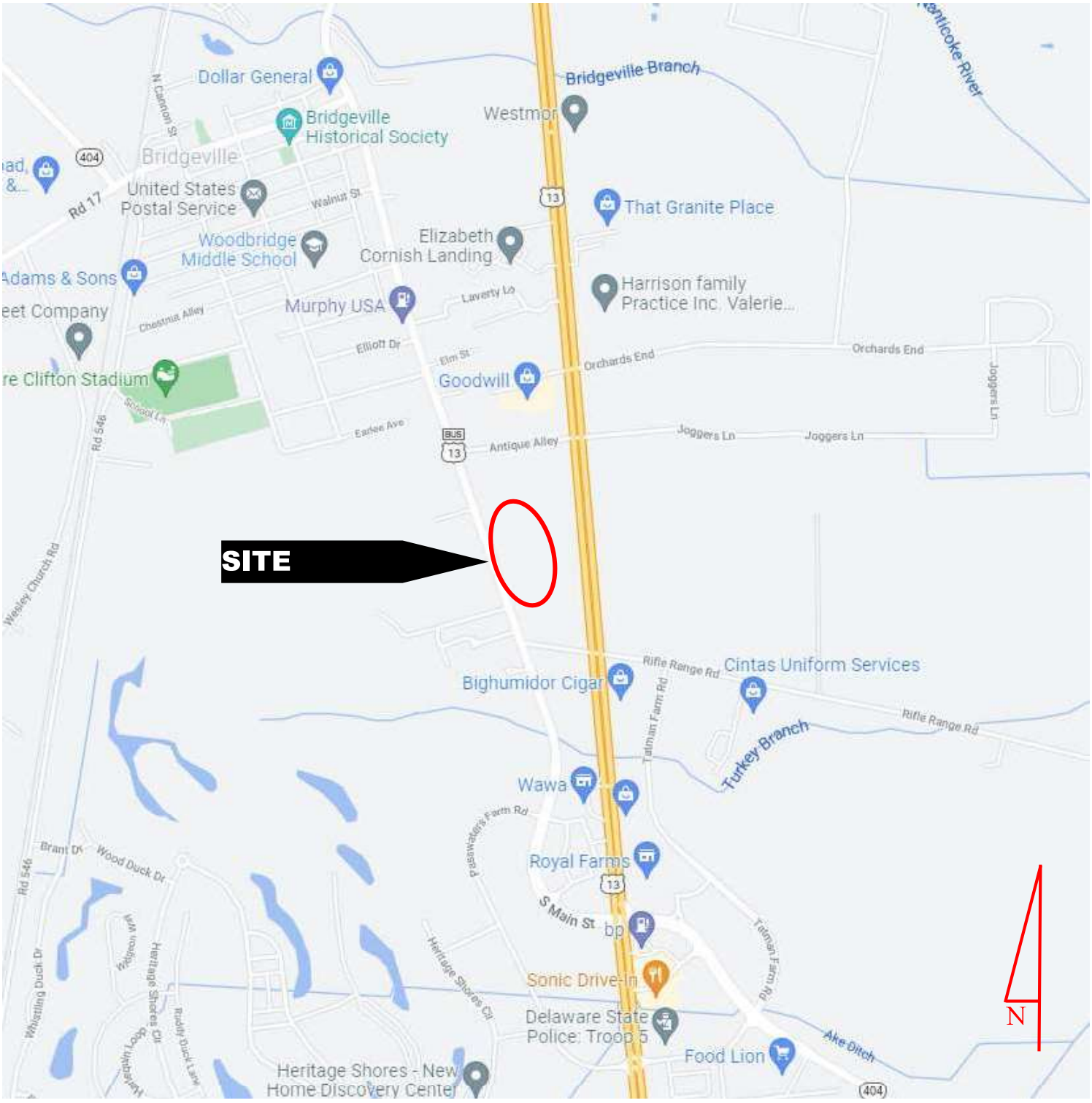
Sincerely,
GEO-TECHNOLOGY ASSOCIATES, INC.

Travis P. Caraway, P.E.
Project Engineer

Gregory R. Sauter, P.E.
Vice President

GRS/TPC/llh
31211931
S:\1 Job File\2021 Projects\31211931-Pods Bridgeville\Report\SWM\PODS Bridgeville - GTA Report for SWM Subsurface Exploration.doc

Attachments: Site Location Plan (1 page)
Exploration Location Plan (1 page)
USDA Soil Survey Map (3 pages)
Sussex County Hydrologic Conditions (1 page)
Subsurface Profile (1 page)
Notes for Exploration Logs (1 page)
Exploration Logs (5 pages)
Infiltration Logs (5 pages)
Particle Size Distribution Report (1 page)
GBA – Important Information about your Geotechnical Engineering Report (2 pages)

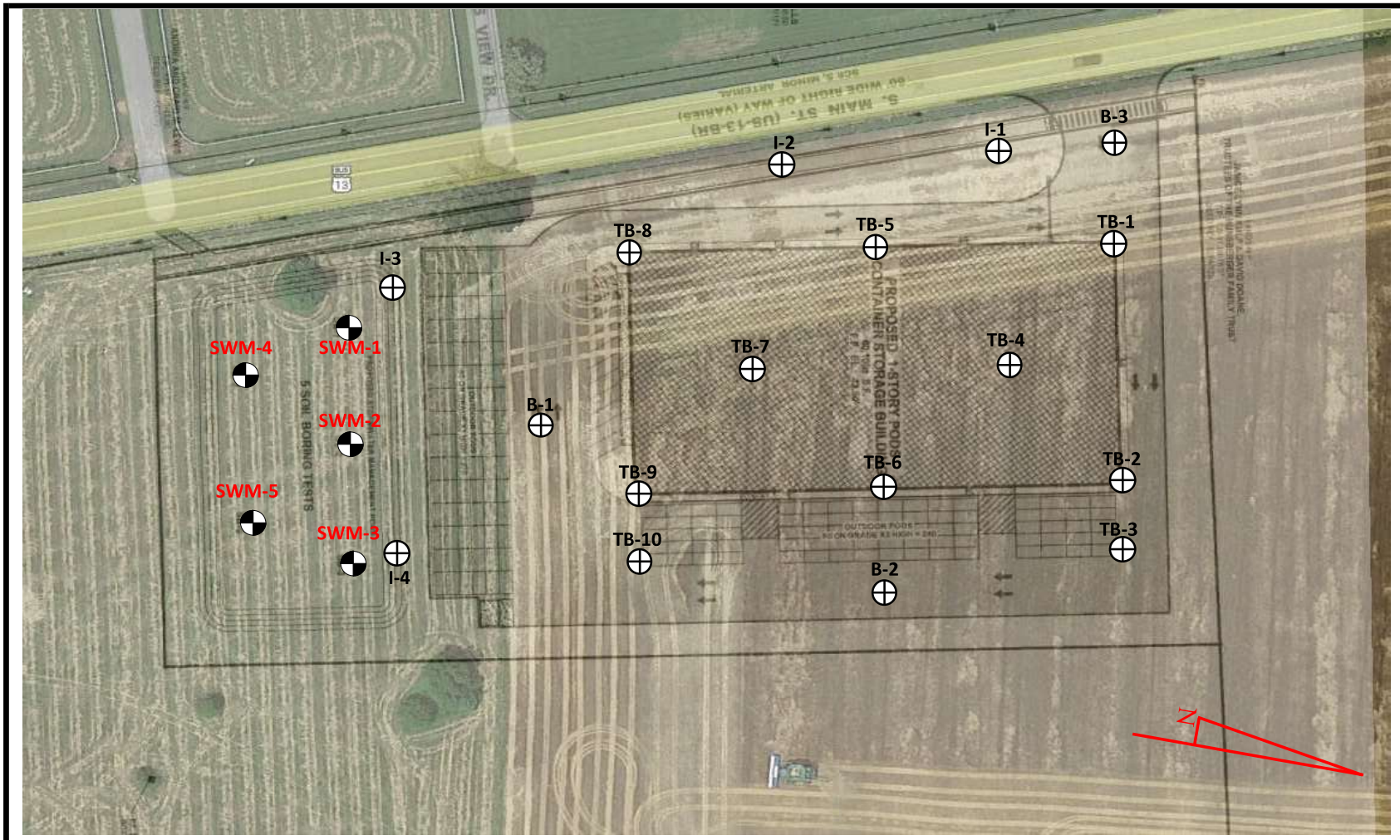


Site Location Plan taken from Google Maps

GEO-TECHNOLOGY ASSOCIATES, INC.
GEOTECHNICAL AND ENVIRONMENTAL CONSULTANTS
21133 Sterling Avenue, Suite 7
Georgetown, Delaware 19947
(302) 855-9761 Fax (302) 856-3388

Site Location Plan
PODS Bridgeville
Bridgeville, Delaware

SCALE	DATE	DRAWN BY	DESIGN BY	REVIEW BY	JOB NO.
NTS	March 2022	GTA	Google Maps	GRS	31211931



Exploration Location Plan taken from Google Earth and a plan titled *PODS STORAGE FACILITY - Proposed Soil Borings Test Locations* prepared by Becker Morgan Group and revision dated January 4, 2022.

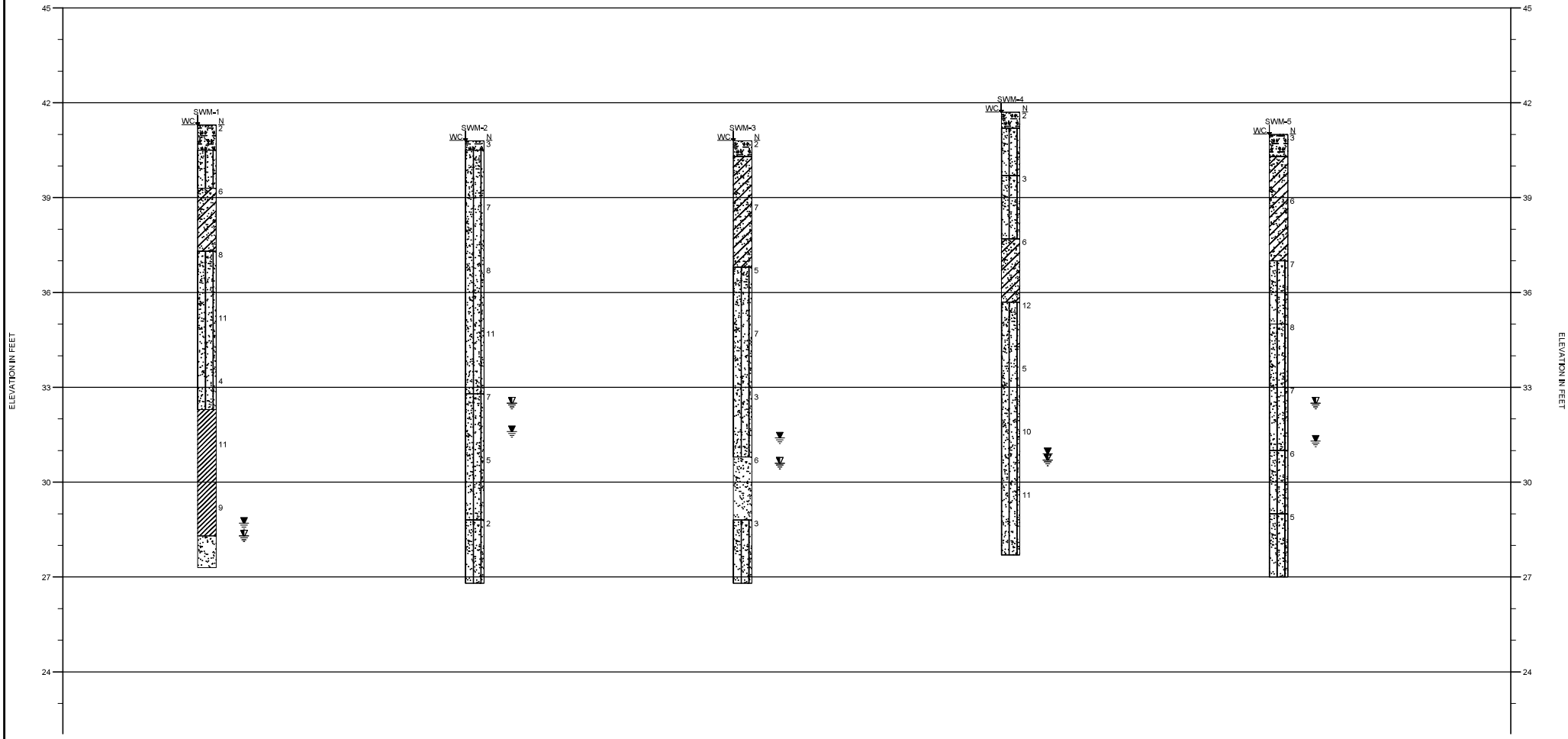
Boring Location

Previous Boring Location

GEO-TECHNOLOGY ASSOCIATES, INC.
GEOTECHNICAL AND ENVIRONMENTAL CONSULTANTS
21133 Sterling Avenue, Suite 7
Georgetown, Delaware 19947
(302) 855-9761 Fax (302) 856-3388

Exploration Location Plan
PODS Bridgeville
Bridgeville, Delaware

SCALE 11x17	DATE	DRAWN BY	DESIGN BY	REVIEW BY	JOB NO.	Figure
1" ~ 35'	March 2022	GTA	BMG	GRS	31211931	2



Symbol Description

- Topsoil
- SM - Silty Sand
- SC - Clayey Sand
- CL - Lean Clay
- SP - Poorly Graded Sand
- SP-SM - Poorly Graded Sand with Silt
- Approximate horizontal location of exploration
- Water table during drilling
- Water table at completion
- Water table at 3rd check

Geo-Technology Associates, Inc.			
SUBSURFACE PROFILE			
VERTICAL SCALE: 1"=3'	DRAWN BY WLG	APPROVED BY TPC	DATE: 3/11/2022
PODS Bridgeville Sussex County, Delaware			
PROJECT NO. 31211931			FIGURE 3

Soil Map—Sussex County, Delaware
(Pods)

Map Scale: 1:1,460 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 20 40 80 120

Feet
0 50 100 200 300

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Sussex County, Delaware
Survey Area Data: Version 22, Aug 26, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 1, 2020—Oct 1, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
HbA	Hambrook sandy loam, 0 to 2 percent slopes	6.4	100.0%
Totals for Area of Interest		**6.4**	**100.0%**